

Liner Trays

Extension of the current design method according to EN 1993-1-3

The project has received financial support from the European Community (RFCS programme) under grant agreement No 754092.



Liner trays

- Typically used for inner sheets of wall systems
- + Cold formed steel profile with wide flange, two webs and two narrow flanges
- Narrow flanges should be restrained by attached profiled steel sheeting
- Flanges and webs are reinforced with stiffeners
- 🔸 Width: 500 600 mm
- 🔸 Height: 90 240 mm





Liner trays

Application for hall-like buildings





Liner tray wall systems

- Conventional solution
 with trapezoidal profile
- New solutions with distance profiles and sidings





- Chapter 10.2, Liner trays restrained by sheeting
 - Application range of liner trays





- Chapter 10.2.2.2, Wide flange in tension
 - **Step 1:** Determine the centroid of the whole cross-section
 - Step 2: Determine the effective width of the wide flange b_{u,eff}



Source: Fig. 10.11 of the EN 1993-1-3



- Chapter 10.2.2.2, Wide flange in tension
 - Step 3: Determine the effective areas of all the compression elements, based on values of the stress ratio $\psi = \sigma_2 / \sigma_1$



Source: Fig. 10.11 of the EN 1993-1-3



Chapter 10.2.2.2, Wide flange in tension

 Step 4: Determine the centroid of the effective cross-section and the moment resistance M_{b,Rd}:





Design by testing according to EN 1993-1-3

- ANNEX A.2 Tests on profiled sheets and liner trays
 - 4 types of tests
 - 1) Single span test
 - 2) Double span test
 - 3) Internal support test
 - 4) End support test

- In Germany, there are additions to the test procedures according to EN 1993-1-3 and DIN 18807-2:
 - "Supplementary test principles for liner tray profiles"







Profile	Test	Nominal thickness t _N [mm]	Span L [mm]	Fixation distance s ₁ [mm]
Liner trays: 110/600 and 160/600 + Trapezoidal profile: 35/207-0.75	Single span test: pressing load	0.75 and 1.00	6000	621
				1242
				1863
				> 6000 (without fixation)
	Internal support test: lifting load	0.75 and 1.00	2000	621
				1242
				1863
				> 2000 (without fixation)



+ Failure mode buckling of the upper flange in the middle of the span







 Failure mode liner tray without outer cladding, load distribution in the lower flange





 Failure mode liner tray without outer cladding, load distribution in the upper flange, lateral buckling





Ultimate bending moment depending on s1



- + Reason:
 - The actual design rule taking into account the effect of the fixing distance s₁ is rather conservative, and furthermore limited to a maximum fixing distance s₁ = 1000 mm.

+ Subject:

- + Influence of the fixing distance s₁ on the bending moment capacity of liner trays
- Improvement of the coefficient b_b and extension of the application range
- \rightarrow Proposal for new calculation for b_b as a function of fixing distance s₁



+ Ideas:

- The ultimate positive bending moment is generally limited by the ultimate compression forces of the narrow flanges
- The compressed flanges are stabilized against lateral buckling by the connections between liner tray and outer cladding
- The ultimate bending moment is approximately proportional to the compression resistance of the narrow flanges
- The reduction coefficient b_b to respect the fixing distance s₁ corresponds approximately to the reduction of the compression resistance of the narrow flanges



Proposed change:

 The (unknown) ultimate bending moment for a liner tray with the fixing distance s_{1,2} (1000 mm < s_{1,2} ≤ 2000 mm) can be determined as follows:

$$M_{c,Rk,2} = M_{c,Rk,1} \cdot \frac{N_{Rk,2}}{N_{Rk,1}} = M_{c,Rk,1} \cdot \beta_b$$

+ with:

- ★ $M_{c,Rk,1}$: (already known) ultimate positive bending moment of the liner tray with a fixing distance s_{1,1} ≤ 1000 mm



+ Procedure:

• Step 1: Determination of the gross cross section of the compressed flanges





+ Procedure:

• Step 2: Determination of the effective cross section of flanges





$$K_R = \frac{E \cdot t^3}{12 \cdot (1 - \mu^2)} \cdot \frac{3}{b_2 \cdot (b_2^2 + b_2 \cdot H)}$$



Procedure:

 Step 3: Determination of the ultimate compression force of narrow flange with respect to lateral buckling

Elastically bedded beam under compression with respect to lateral buckling:

$$K_{fz} = \frac{E \cdot t^3}{12 \cdot (1 - \nu^2)} \cdot \frac{6}{2 \cdot h^3 + 3 \cdot b \cdot h^2}$$

Calculation of the critical axial force:

$$N_{cr} = \frac{n^2 \cdot \pi^2 \cdot E \cdot I_{fz}}{s_1^2} \cdot \frac{K_{fz} \cdot s_1^2}{n^2 \cdot \pi^2}$$



+ Procedure:

Step 3: Determination of the ultimate compression force of narrow flange with respect to lateral buckling

Calculation of the characteristic compression forces N_{Rk} , i, $s_{1,1}$ and N_{Rk} , i, $s_{1,2}$ for the fixing distance $s_{1,1}$ and $s_{1,2}$:

$$N_{Rk} = \chi(a_0) \cdot A_{fz} \cdot f_{yb}$$

Iteration of the ultimate compressive stress:

$$\sigma_k = \chi(a_0) \cdot f_{yb}$$



+ Procedure:

Step 4: Determination of the reduction coefficient b_b

The reduction coefficient b for the distance $s_{1,2}$ is thus calculated as follows:

$$\beta_b = \frac{N_{Rk,1} , s_{1,2} + N_{Rk,2} , s_{1,2}}{N_{Rk,1} , s_{1,1} + N_{Rk,2} , s_{1,1}}$$



Summary

- + Conventional liner tray wall systems do not fulfil the actual energy requirements.
- New solutions are developed for which no calculation methods are available in current regulations, or application boundaries of existing methods are exceeded.
- Within the scope of GRISPE, extensive experimental investigations have been performed on liner trays for fixing distances that are normatively not or insufficiently covered.
- Practicable calculation methods have been derived based on existing regulations and methods.



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